

Seismic Forces Acting on Electrical Distribution Systems

When subjected to an earthquake, electrical distribution systems must resist lateral and axial buckling forces, and the restraint components for these systems must resist pullout and localized structural failures.

Most electrical distribution systems are suspended from the deck above on fixed hanger rod systems. They may be supported singly or there may be several pieces of conduit or buss ducts attached to a common trapeze. On some occasions the conduit may run vertically or may be mounted to the floor.

Suspended Systems

Most codes do not require that electrical distribution supported on non-moment generating (swiveling) hanger rods 12 in or less in length be restrained. The 12 in length was determined based on the natural frequency of systems supported on the short hanger rods. In practice, it has been found that the vibrations generated by earthquakes do not excite these types of systems and, although the systems move back and forth somewhat as a result of an earthquake, they do not tend to oscillate severely and tear themselves apart.

There are also exclusions in most codes for small pieces of conduit, no matter what the hanger rod length. Again, the basis for this exclusion is based on the post-earthquake review of many installations. It has been found that smaller conduit runs are light and flexible enough that they cannot generate enough energy to do significant damage to themselves.

For cases where restraints are required, however, the forces involved can be significant. This is due to the difference between the spacing of the system supports and their restraints. Supports for these systems are typically sized to carry approximately a 10 ft length of conduit or duct (in the case of trapezes, multiple pieces of conduit each approx 10 ft long). Seismic restraints, on the other hand, are normally spaced considerably further apart with the spacing varying by restraint type, restraint capacity, conduit size, and the seismic design load. It is very important to be aware of the impact of the difference in spacing as the wider this spacing, the larger the seismic load when compared to the support load. Guidance in determining restraint spacing requirements is available in Chapter D4 of this manual. (Note when using these tables that conduit should be assumed to be similar in weight and performance to the equivalent pipe size.)

To illustrate this difference, consider a simple example of a single piece of conduit weighing 50 lb/ft being restrained against a 0.2g seismic force with restraints located on 80 ft centers and supports located on 10 ft centers. The load that is applied to the hanger rods by the weight of the conduit is 50 lb/ft x 10 ft or 500 lb each (assuming single rod supports). The horizontal load that occurs at the restraint locations is the total restrained

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weight (50 lb/ft x 80 ft = 4000 lb) multiplied by the seismic force (0.2g) or 800 lb. Thus the seismic load is larger than the vertical dead load.

Restraints for suspended systems are normally in the form of cables or struts that run from the conduit up to the deck at an angle. Because of the angle, horizontal seismic loads also generate vertical forces that must be resisted. Therefore, restraint devices must be attached at support locations so that there is a vertical force-resisting member available.

As the angle becomes steeper (the restraint member becomes more vertical), the vertical forces increase. At 45 degrees the vertical force equals the horizontal force and at 60 degrees the vertical force is 1.73 times the horizontal force.

The net result is that for cable systems or for struts loaded in tension, the uplift force at the bottom end of the restraint can be considerably higher than the downward weight load of the conduit. Returning to our example, assume that we have a restraint member installed at a 60 degree angle from horizontal and that the lateral force will load it in tension. In this case, the 800 lb seismic force generates an uplift force of 1.73×800 lb or 1384 lb. This is 884 lb more than the support load and, depending upon the support rod length and stiffness, can cause the support rod to buckle. Rod stiffeners are used to protect against this condition and sizing information is available in Chapter D4 of this manual.

Unlike cables, if struts are used for restraint they can also be loaded in compression. In the example above, if the strut were loaded in compression, the 1384 lb load would be added to the support load (trying to pry the hanger rod out of the deck). The total support capacity required would be 1384 lb + 500 lb or 1884 lb. As a consequence, when using struts, the hanger rod must be designed to support 1884 lb instead of the 500 lb maximum generated with cables. Hanger rod sizing information is also available in Chapter D4 of this manual.

Riser Systems

Where conduit is running vertically in structures, except for the loads directly applied by vertical seismic load components identified in the code, there will be little variation in vertical forces from the static condition. Lateral loads are normally addressed by local anchorage and the spacing between these anchors is not to exceed the maximum tabulated lateral restraint spacing indicated in the design tables in Chapter D4.

Floor-Mounted Systems

The primary difference between floor- and ceiling-mounted electrical distribution systems is that the support loads in the distribution system support structure are in compression instead of tension (as in the hanger rods). Although a support column and diagonal

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cables can be used, a fixed stand made of angle or strut is generally preferred. Rules relating to restraint spacing and the sizing information for diagonal struts are the same as for hanging applications.

However, the support legs need to be designed to support the combined weight and vertical seismic load (for a two-legged stand and the example above, $500 \text{ lb} / 2 + 1384 \text{ lb}$ or 1634 lb) in compression (Note: $500 \text{ lb} / 2$ is the load per leg for two legs). The anchorage for the legs needs to be able to withstand the difference between the dead weight and the vertical seismic load (in the example above $1384 \text{ lb} - 500 \text{ lb} / 2$ or 1134 lb).

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